

The Application of Biofiltration with Polyurethane Foams for Domestic Sewage Treatment

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ABSTRACT

The study presents the results of an experimental method for purification of household sewage on the biofilters filled with polyurethane waste in the form of trims of upholstery foam (PUF). We assessed the removal effectiveness of organic compounds, ammonium nitrogen and total suspended solids from sewage pre-treated in a septic tank at hydraulic loading ranging from 31.8 to 229.2 mm⁻¹. The results of five month study identified the biofilter filled with 30 cm thick foam as the most effectively removing BOD₅, COD_{Cr}, N-NH₄⁺ and total suspended solids. Average reduction in pollutant levels for this filter reached 95%, 94%, 84% and 68%, respectively. PUR foam in the form of trims of upholstery foam filling the vertical flow filters ensured favorable conditions for the development of heterotrophic and nitrifying bacteria without any need for additional aeration.

Keywords: waste, polyurethane foam, sewage treatment, biofilter

INTRODUCTION

For a long time now, fast developing countries have faced an issue of untreated household sewage discharged directly to the environment. An inadequate infrastructure for water supply and sewage removal increases the risk of diseases spreading amongst the people exposed to unsafe water. In 2015, the World Health Organization reported that 127 million people used basic services, 263 million used limited services, 423 million used unimproved sources and 159 million used surface water [UNICEF, WHO...2017]. The insufficient hygiene and sanitation related to the above-mentioned problem resulted in million people deaths. The access to safe water and sanitation are essential to human health and to the environmental sustainability and economic prosperity.

With the increasing cost of electricity, it is important to conduct sustainable wastewater treatment process management. For aerobic wastewater treatment with activated sludge the aeration process may consume up to 60% of the total WWTP power demands [Gikas 2017].

In the case of on-site sewage treatment, effective and economical solutions must be proposed. Easy to use technologies that have low energy demand should be used. The technologies that do not require energy inputs e.g. for additional aeration, can be an example.

One of the popular on-site sewage treatment systems is a combination of a septic tank and a vertical flow sand filter [Chmielowski and Ślizowski 2008, Wąsik and Chmielowski 2017, Dacewicz and Chmielowski 2018, Wąsik and Chmielowski 2018]. It yields good results and is inexpensive but its main disadvantage is the possible silting up of the filter bed. Sand-filled filters are also poorly aerated due to a low content of the empty fraction.

In order to increase the removal rate of organic and biogenic compounds and to prevent the filter from silting up, the sand filling may be replaced, partly or completely, with porous materials. The adsorption capacity of porous materials depends on an internal system of pores of different sizes. Natural porous materials commonly used to treat water and sewage include clay [de Gisi et al. 2016, Dacewicz and Chmielowski

2018], chitosan [Ali 2012, de Gisi et al. 2016] and zeolite [Ali 2012, de Gisi et al. 2016, Dacewicz and Chmielowski 2018].

Natural materials may be replaced with plastic fillings that feature equally large specific surface area but tend to clog less. An interesting solution is using materials of porosity exceeding 90%, e.g., foams made of polyurethane or polyethylene. In the 1990s, professor Hideki Harada et al. from the Nagaoka University of Technology in Japan used a downflow hanging sponge (DHS) system for sewage post-treatment. Sponge cubes hanging freely in the air, called “first-generation DHS” were used to treat the effluents from an anaerobic reactor UASB. To date, extensive studies focused on the use of six generations of the DHS technology for treatment of low strength wastewater [Tawfik et al. 2008, Tawfik et al. 2010, Tawfik and Klapwijk 2010, Miyaoka et al. 2017]. In order to prevent the deformation of the flexible foams and their adhering to each other, they were packed in plastic sheaths, starting with the DHS-G3 generation. The sixth generation systems use rigid polyethylene foams manufactured by copolymerization with an epoxy resin [Onodera et al. 2014].

DHS reactor has many advantages but it cannot be used for treatment of raw sewage containing high levels of suspended solids (SS). Their presence in fluids flowing through the foams prevents oxygen and substrates from supplying the biofilm [Guo et al. 2010]. Therefore, it is recommended to remove larger suspended matter and solids before the sewage enters the DHS reactor. Uemura et al. [2012] investigated the direct treatment of settled sewage using an independent DHS system. They used foams with pores of different sizes, enabling the reduction of COD by 83–85% \pm 12%, and of ammonium nitrogen by 90–95% \pm 13%. Ehsas [2013] used a laboratory-scale in-

dependent DHS system to treat synthetic sewage (tap water with dog feed added). With cylindrical polyurethane foams randomly packed into the reactor modules, the pollution reduction ranged from 56–76% for COD, 58–91% for BOD₅, 83–100% for ammonium nitrogen, and 58–97% for total suspended solids. Tawfik et al. [2011] used the DHS system with randomly packed foams to treat gray sewage. They achieved the removal of COD, N-NH₄⁺ and total suspended solids at a level of 90–96% \pm 2%, 26.7–86% \pm 13% and 93.5–98.2% \pm 1%, respectively.

This article discusses a possibility of using plastic waste in the form of polyurethane foams for purifying household sewage after pre-treatment in a primary sedimentation tank. It also presents a correlation between the filter thickness and effective removal of organic compounds, nitrogen compounds (in form of ammonium nitrogen) and total suspended solids on biofilters working under variable hydraulic loading.

MATERIAL AND METHODS

The biofilters were filled with trims of flexible polyurethane foam of random shapes, supplied by Eurofoam Polska (Fig. 1), used commercially as upholstery foam. They featured high porosity of 94.6% [Dacewicz 2018a].

Figure 2 shows a granulometric curve plotted based on the weight of foam pieces retained on individual sieves of a shaker for loose soils. Nominal diameters, d_{10} and d_{60} , read from the curve, of 4 mm and 10 mm (similar to a gravel) (PN_EN ISO 14688–1), respectively, were used to calculate the coefficient of grading non-uniformity ($d_{60}/d_{10} = 2.5$).



Figure 1. Pieces of polyurethane foam waste used as biofilter filling (phot. E. Dacewicz)

The experimental filters were sectional models of a vertical flow filter, studied on a semi-technical scale. The experimental model consisted of three identical PVC columns, of 10 cm in diameter and 100 cm high. The layers of filling in individual columns were 60 cm, 30 cm, and 15 cm thick. A grate supporting the bed was installed at the bottom of each filter layer. Figure 3 shows a diagram of the column model.

The test sewage was first pretreated in a septic tank and then transferred to a storage tank for further reduction of BOD_5 and total suspended solids. The effluents from the storage tank were fed onto the top foam layer, and then flowed down gravitationally. The sewage was dosed onto the model columns with peristaltic pumps started cyclically every hour (24 doses per day). Each column in the experimental model was subjected to the same hydraulic loading during one of seven stages that changed every three weeks during five months of the experiment. The hydraulic loading was applied in the following variants: $31.8 \text{ mm}\cdot\text{d}^{-1}$ (II and IV stage), $76.4 \text{ mm}\cdot\text{d}^{-1}$ (I and V stage), $114.6 \text{ mm}\cdot\text{d}^{-1}$ (III and VI stage) and $229.2 \text{ mm}\cdot\text{d}^{-1}$ (VII stage). The acclimation phase of the beds lasted for 1.5 week before the actual experiment. The pretreated sewage was collected from a separating chamber downstream from the storage tank, and the treated sewage was collected as filtrates from individual columns of the model. The sewage samples underwent physi-

cal and chemical analyses for the following indicators of sewage pollution: dissolved oxygen, BOD_5 , COD_{Cr} , total suspended solids, and ammonium nitrogen. The physical and chemical analyses were performed 2–3 times a week, on average, for 8–9 measurement series.

Filter performance was determined based on measured BOD_5 , COD_{Cr} , ammonium nitrogen ($N-NH_4^+$), and total suspended solids. The values for these indicators were determined as per relevant standards: EN 1899–1:2002 for BOD_5 , ISO 6060:2006 for COD_{Cr} , PN-C-04576–4:1994 for ammonium nitrogen, and EN 872:2007 for total suspended solids.

The statistical analyses were performed with the STATISTICA 12.5 software package. Mean levels of the studied indicators in the sewage collected from specific layers of the filter bed were used in the ANOVA analysis. The values of analyzed sewage pollutants (BOD_5 , COD_{Cr} , total suspended solids, and ammonium nitrogen) from individual levels of the filter bed were used as a dependent variable. Before ANOVA, the Grubbs' test for outliers must be performed and the normality of variables distribution must be established, which is then verified with the Shapiro-Wilk test. Significant differences between variations of removed pollutants were verified with an analysis of homogeneity of variances using the Levene's test. In order to establish which intergroup differences in the reduction rate were of the highest sig-

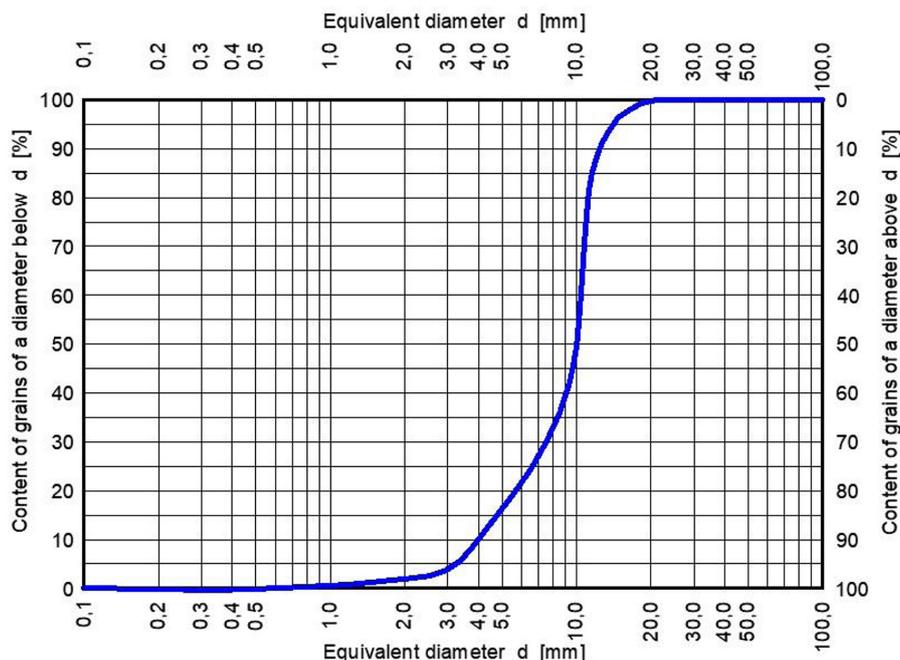


Figure 2. Granulometric curve for filter filling of PUR foam

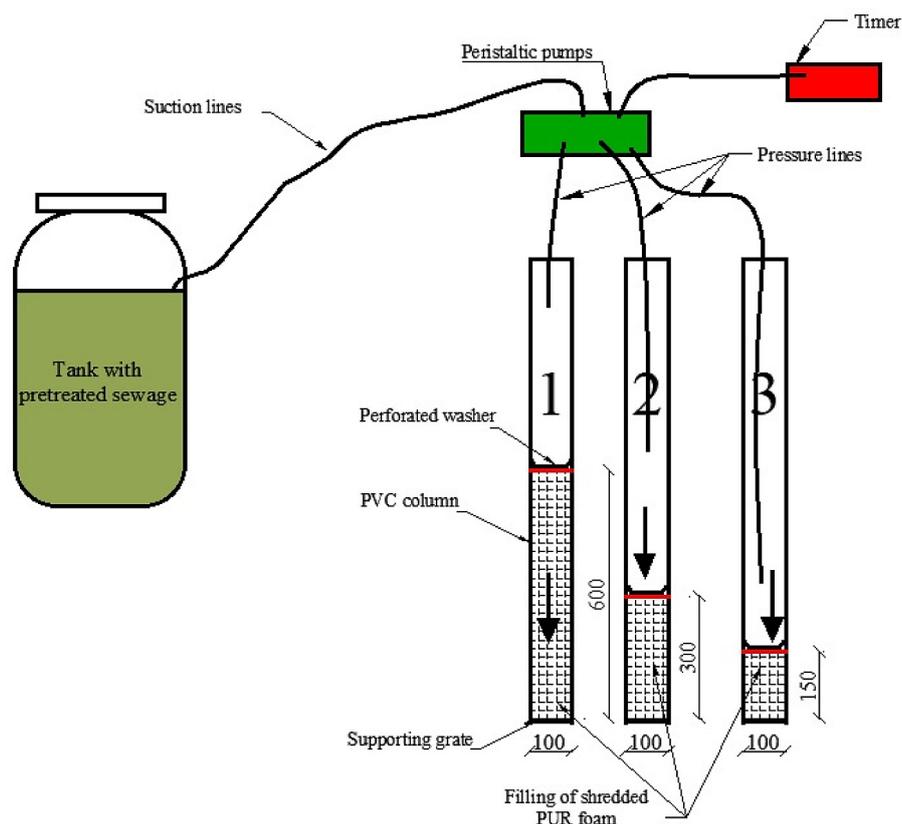


Figure 3. Diagram of a column model

nificance, the non-parametric Kruskal-Wallis test was performed. A difference in means for a given pair of groups was considered significant when the probability value (p-value) was below 0.05.

RESULTS AND DISCUSSION

Table 1 presents the basic descriptive statistics for household sewage post the septic tank, supplied to the storage tank and then to the test station. The sewage flowing into the experimental model varied greatly in BOD_5 ($WN=1.03$). The values of that indicator ranged from $20.0 \text{ mgO}_2 \cdot \text{dm}^{-3}$ to $760.0 \text{ mgO}_2 \cdot \text{dm}^{-3}$. Mean BOD_5 was $165.6 \text{ mgO}_2 \cdot \text{dm}^{-3}$, and standard deviation reached $170.1 \text{ mgO}_2 \cdot \text{dm}^{-3}$. The quoted values demonstrate a significant variation in quality of supplied sewage, with the spread of $740.0 \text{ mgO}_2 \cdot \text{dm}^{-3}$.

At the highest hydraulic loading, the short stay of sewage in the storage tank did not result in sedimentation of total suspended solids, and thus in the reduction of BOD_5 . Mean COD_{Cr} values for pretreated sewage reached $272.5 \text{ mgO}_2 \cdot \text{dm}^{-3}$. This indicator was more stable than BOD_5 ($WN=0.65$) and ranged from $98.0 \text{ mgO}_2 \cdot \text{dm}^{-3}$ to $972.0 \text{ mgO}_2 \cdot \text{dm}^{-3}$.

The mean value of COD_{Cr}/BOD_5 ratio in the sewage fed onto individual columns was 1.64, and this implies the presence of organic pollutants both difficult and easy to degrade in biochemical processes [Henze et al. 2008]. In their study of pretreated sewage in a septic tank [Wąsik and Chmielowski 2013, Wąsik and Chmielowski 2017, Wąsik et al. 2017, Dacewicz et al. 2018] obtained similar mean values of these indicators.

Sewage post the storage tank, in which the effluents from the septic tank underwent additional sedimentation of suspended organic matter, showed low values of total suspended solids. When compared to the values for total suspended solids determined directly after the septic tank, they were lower by 20%, on average. The concentration range for suspended solids fluctuated from $37.0 \text{ mg} \cdot \text{dm}^{-3}$ to $293.0 \text{ mg} \cdot \text{dm}^{-3}$, while the mean value for this indicator was $93.4 \text{ mg} \cdot \text{dm}^{-3}$.

The values of the above-mentioned indicators reported by other authors [Wąsik and Chmielowski 2013, Dacewicz 2018] imply higher concentrations of organic compounds and total suspended solids in sewage directly at an outlet from a septic tank.

The sewage flowing into the experimental model after storage in the storage tank

Table 1. Quality characteristics of pre-treated sewage flowing into model columns

Indicator	Unit	Basic descriptive statistics					
		Mean value	Median	Maximum value	Minimum value	Standard deviation	Coefficient of variation
Dissolved oxygen	mgO ₂ ·dm ⁻³	0.22	0.14	0.90	0.01	0.21	0.98
COD _{Cr}	mgO ₂ ·dm ⁻³	272.5	229.0	972.0	98.0	177.2	0.65
BOD ₅	mgO ₂ ·dm ⁻³	165.6	100.0	760.0	20.0	170.1	1.03
Suspended solids	mg·dm ⁻³	93.4	85.0	293.0	37.0	51.5	0.55
Ammonium nitrogen	mg·dm ⁻³	166.3	162.8	260.5	84.8	34.1	0.21

showed low variation in ammonium nitrogen level (WN=0.21). This value ranged from 84.8 mg·dm⁻³ to 260.5 mgO₂·dm⁻³, and the mean value was 166.3 mgO₂·dm⁻³, which was similar to the value reported by other authors [Wąsik and Chmielowski 2013, Dacewicz 2018b]. When compared to the values reported in other publications for pretreated sewage in the anaerobic UASB reactor [Tawfik et al. 2011, Onodera et al. 2014], the ammonium nitrogen levels in our study were over five times higher.

The data presented in Figure 4 show that the sewage leaving individual columns in the model was characterized by high mean dissolved oxygen (DO) levels, despite very low oxygen content in the sewage supplied to the experimental model (Table 1).

The level of dissolved oxygen in the treated sewage increased with the growing thickness of

the filter layer. This indicates a very good oxygen saturation of the filter bed filled with waste upholstery foam. Uemura et al. [2016] analyzed the oxygen mass transfer in new PUR foams placed in a plastic sheath and observed an increase in DO accompanying an increase in foam thickness to 15–30 cm. For these values of the filter layer thickness, the level of dissolved oxygen rose from zero to 7.5 mgO₂·dm⁻³, while with further 15 cm of foam this value reached only 9 mgO₂·dm⁻³. During five months of the experiment, the increase in the hydraulic loading resulted in a drop in the oxygen content in the effluents from individual columns (Fig. 4). Thus, the mean DO value ranged from 3.2 mgO₂·dm⁻³ to 4.8 mgO₂·dm⁻³ for the filter bed with 60 cm thick foam, and was similar to the values obtained for the layer of 30 cm (2.6 mgO₂·dm⁻³ to 4.8 mgO₂·dm⁻³). For the 15 cm thick foam, the mean DO concentration

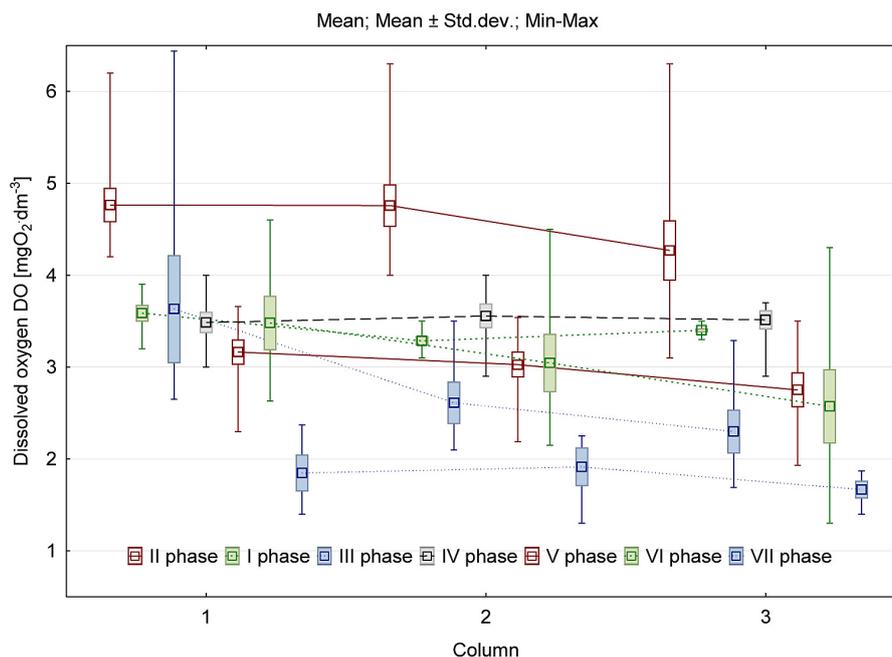


Figure 4.. Box-plot graphs of dissolved oxygen concentrations in sewage from columns 1, 2 and 3

reached 2.3–4.3 mgO₂·dm⁻³. Similar results were reported by Onodera et al. [2014], who observed an increase in the dissolved oxygen level in the DHS reactor post UASB reactor, to a level of 3 mg O₂·dm⁻³ at a distance of 40 cm from the sewage inlet. Moreover, Tawfik et al. (2011) noticed an increase in DO value from 0.8 mg O₂·dm⁻³ to 2.1–3.1 O₂·dm⁻³ in the first segment of the DHS reactor during purification of gray sewage.

The results of non-parametric Kruskal-Wallis test for dissolved oxygen levels in the treated sewage at individual stages of the experiment are presented in Table 2. Only at Stage I, partly considered as the period of biofilter acclimation, the p-value was below 0.05, and this means that the differences in the DO values at the compared stages were significant.

Figures 5–7 present the effectiveness of pollutant removal on individual filters for the analyzed hydraulic loading. Additionally, a part of Stage I of the experimental model operation can be considered as the bed acclimation period. This is implied by a low (17.8–29.6 %) effectiveness of removal of hardly biodegradable organic compounds. In the discussed period, the COD_{Cr} value in the effluents from all three columns was high and exceeded 150 mg mgO₂·dm⁻³. This value is accepted by Polish law for treated sewage for treatment plants below 2000 population equivalent (p.e.) [Regulation...2014]. Additionally, during the highest hydraulic loading of 114.7 mm·d⁻¹ and 229.2 mm·d⁻¹, at mean C/N ratio of 2.8 (III and VI) and 1.9 (Stage VII), we detected higher than acceptable COD_{Cr} value in the effluent from household sewage treatment plant. On one hand, the low effectiveness of COD_{Cr} removal during the highest loading of the beds could result from a presence of hardly degradable organic compounds, as at that stage the mean COD/BOD ratio in raw sewage was 1.9. On the other hand, the high efficiency of removal of easily biodegradable organic compounds indicated an increase in the biomass of heterotrophic bacteria. This could result in blocking of foam pores, leading to a reduced ability for transport of substrates and a

drop in the dissolved oxygen levels (Fig. 4) [Tawfik et al. 2010, Uemura et al. 2016].

The authors observed the highest (73.1%) average COD_{Cr} reduction during filtration of pretreated sewage through the 60 cm thick foam (column 1) at the lowest hydraulic loading of 31.8 mm·d⁻¹ (Fig. 5). Removal of easily degradable organic substances was fairly independent of the fluctuations in hydraulic loading (Stages II–VI) and reached 90%, on average. The highest BOD₅ reduction, at a similar level of 95%, occurred on the beds with the 60 cm and 30 cm thick foam at the hydraulic loading of 114.7 mm·d⁻¹ (Stage III). Column 1, with the increasing hydraulic loading, and consequently oxygen availability, provided more favorable conditions for bed colonization by nitrifying bacteria (Fig. 5). Stage VI contributed to the highest (71.8%) reduction of ammonium nitrogen. Over the entire study period, the removal of total suspended solids on the 60 cm thick foam ranged from 69.2% (Stage VII) to 86.6% (Stage III).

For the 30 cm thick foam, the effectiveness of pollutants removal was more uniform. In column 2 (Fig. 6), the mean rate of the organic compounds removal was the highest for the analyzed thickness levels of PUR waste. At Stage III, COD_{Cr} removal reached the level of 93.8%. Similar high values for new fillings were obtained by Tawfik et al. [2011] for the treatment of gray sewage, and Bundy et al. [Bundy et al. 2017] during filtration of household sewage on foam of different thickness. Reduction of easily biodegradable organic compounds on column 2 ranged from 67.7% (Stage VII) to 94.7% (Stage III). The effectiveness of ammonium nitrogen removal by the 30 cm thick foam fluctuated from 46.8% (Stage VII) to 68.3% (Stage VI). Column 2 was the least effective in the removal of total suspended solids, which ranged from 74.6% (Stage IV) to 84.0% (Stage II).

Elimination of ammonium nitrogen at Stage I depended on the filling thickness, and increased from 42.2% (Fig. 7) to 61.7% (Fig. 5) with an increase in foam thickness. The columns filled with

Table 2. Kruskal-Wallis test results for dissolved oxygen levels in treated sewage during five months of the experiment

Experiment Stage						
I	II	III	IV	V	VI	VII
7.4612 p = 0.0240	2.6678 p = 0.2635	5.4854 p = 0.0644	1.1594 p = 0.5601	2.58 p = 0.2753	3.939 p = 0.1395	1.6886 p = 0.4299

Statistically significant differences are marked in red

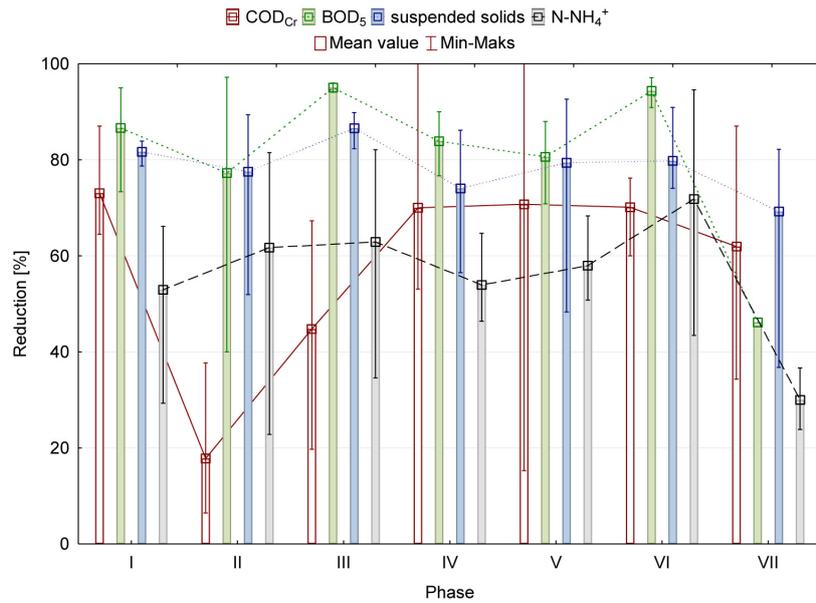


Figure 5.. Reduction of pollution levels in the column 1 for Stages I-VII

the 60 cm and 30 cm thick foam removed ammonium nitrogen in the most efficient way (ca. 70%). High porosity of the polyurethane filling created similar oxygen conditions in these filters and the conditions for mass transfer (substrate, nutrients) which were favorable for growth of nitrifying bacteria [Uemura et al. 2016].

The sewage quality indicators were the most variable in filtrates leaving the column filled with foam of the lowest thickness (column 3). A significant drop in the effectiveness of organic compounds removal on column 3 following an increase in hydraulic loading resulted from a too

short time of pollutants trapping on the 15 cm thick layer of foam. At two initial stages of their operation, COD_{Cr} and BOD₅ removal reached only 8% and 34.4%, respectively. The process of ammonium nitrogen removal, with efficiency at a level of 60%, did not depend on the fluctuations in hydraulic loading (Fig. 7). The lowest biomass increase found for this foam thickness resulted in the least effective elimination of organic pollutants – on average, 70.8% and 89.3% for COD_{Cr} and BOD₅, respectively. The greatest rate of organic suspended matter removal on the 15 cm thick foam can be explained by the unfa-

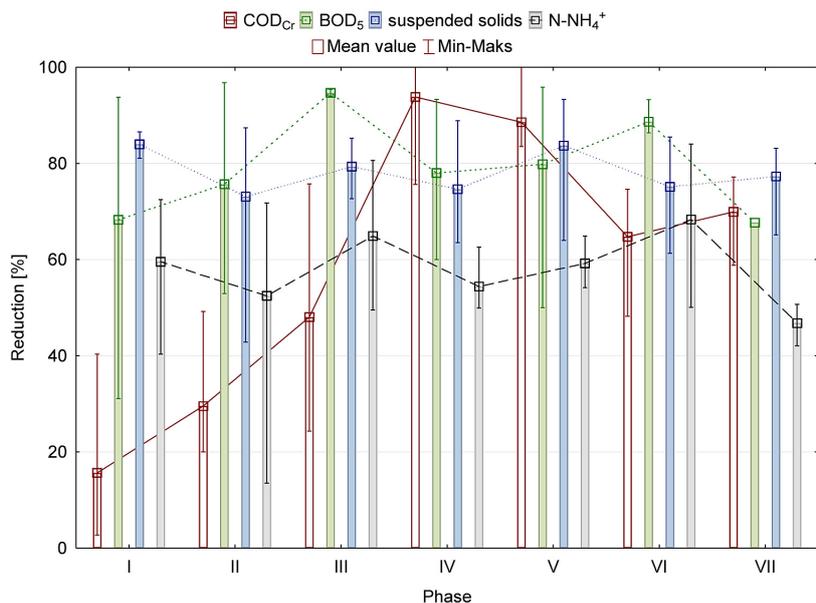


Figure 6. Reduction of pollution levels in the column 2 for Stages I-VII

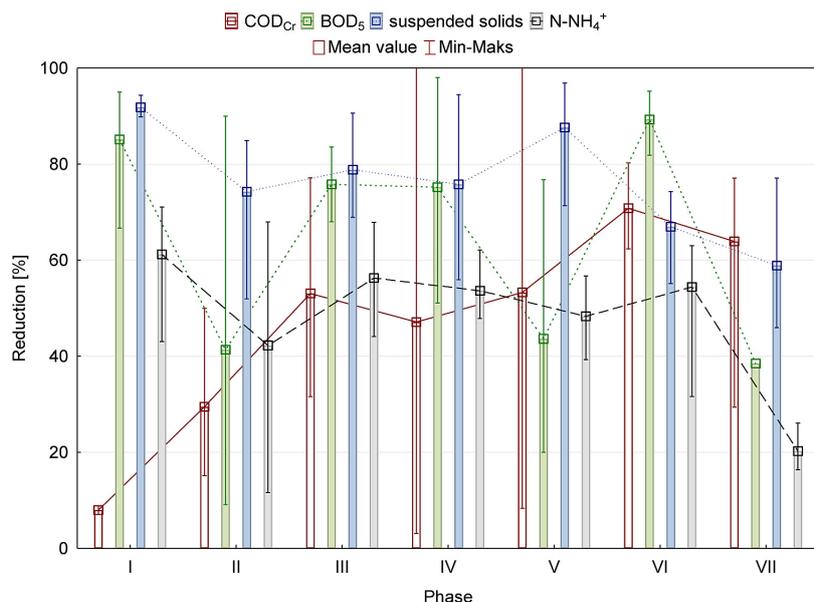


Figure 7. Reduction of pollution levels in the column 3 for Stages I-VII

favorable conditions for biomass development. The biomass present in the filtrate consisted in dead bacteria washed out from the bed. The COD_{Cr} to BOD₅ ratio in the treated sewage, ranging from 3.8 to 18.7 for 60 cm bed, from 4.0 to 11.1 for 30 cm bed, and from 3.1 to 8.1 for 15 cm bed, indicated only a presence of chemical compounds that could not be degraded with biological methods during five months of the experiment.

Waste polyurethane foam was similarly effective in the treatment of household sewage downstream of the primary sedimentation tank during five months of the experiment, as the DHS technology coupled with the anaerobic UASB reactor. The rate of pollutant removal in a system of new sponges, a so-called “curtain” (DHS of second generation), was 95%, 84% and 61% for BOD₅, COD and ammonium nitrogen, respectively [Ehsas 2011]. In the third and the fourth generation of the DHS system, Tawfik et al. [2011] achieved COD, BOD₅ and ammonium nitrogen reduction at the level of 95%, 98% and 86%, respectively.

In their studies, the N-NH₄⁺ level in the sewage treated in the above-mentioned system was ca. five times lower than in our experiments with PUR waste. Doma et al. [2014] showed that the integrated system UASB-DHS reduced COD by 90% and BOD by 95% in process of treatment of catering wastewater.

The last stage of the study investigated a relationship between the bed filter thickness and the differences in the values of BOD₅, COD_{Cr}, total suspended solids and ammonium nitrogen in the treated sewage leaving individual columns of the experimental model. Table 3 presents the results of non-parametric Kruskal-Wallis test for variables that lack normal distribution and show non-homogeneous variance. The performance of the 30 cm thick foam was the most stable during five months of the research under the variable hydraulic load. We observed no significant differences in elimination of BOD₅, N-NH₄⁺ and total suspended solids at significance level $\alpha=0.05$ for this filling thickness.

Table 3. Kruskal-Wallis test results during five months of the experiment (Stages I-VII)

Bed thickness	Oxygen level	Reduction			
		COD _{Cr}	BOD ₅	N-NH ₄ ⁺	Suspended solids
60 cm	29.5075 p = 0.00005	17.116 p = 0.0089	9.493 p = 0.1477	16.1302 p = 0.0131	4.5259 p = 0.6059
30 cm	30.7559 p = 0.00003	28.9216 p = 0.00006	4.2732 p = 0.6398	11.5956 p = 0.0716	6.5072 p = 0.3688
15 cm	35.1651 p = 0.00000	13.4368 p = 0.0366	11.0707 p = 0.0862	20.3743 p = 0.0024	15.2456 p = 0.0184

Statistically significant differences are marked in red

For the analyzed three levels of thickness, significant differences were found for COD_{Cr} removal, and, additionally, for elimination of ammonium nitrogen for 60 cm bed. This could result from an insufficient level of oxygen saturation and from poorer gas transfer conditions associated with the too high level of foam in column 1. For the lowest foam thickness (15 cm), the probability value, p , exceeded 0.05 only for easily biodegradable compounds, and this means that the differences in BOD_5 reduction at compared stages were insignificant.

The data presented above demonstrate a significant correlation between the thickness of the filter bed and the COD_{Cr} value in the treated sewage. This relationship does not apply to mean BOD_5 values in the treated sewage during five months of the experiment. This may indicate the highest intensity of aerobic processes in the initial layers (15 cm).

CONCLUSIONS

1. Removal of carbon compounds on a filter filled with polyurethane waste proceeded with high intensity. The BOD_5 elimination rate was high already for the bed with 15 cm thick filter layer, and amounted to 89.3% on average. Tawfik et al. [2011] made similar observations for a DHS system coupled with an anaerobic UASB reactor. During an analysis of a sewage profile along the DHS system height they noticed that the upper part of the DHS system (first and second segment) was mainly responsible for COD removal, and this resulted in lower organic loading and less intense competition for oxygen when autotrophic nitrifying bacteria were also present. This meant that nitrification ran smoothly in the successive (third) segment of the DHS system.
2. The elimination rate of easily biodegradable organic substances increased along with the filter layer thickness. The biofilter filled with foam of the greatest thickness, 60 cm (column 1), was the most effective in BOD_5 reduction, which was on average 95.0%. Removal of the organic substances difficult to degrade biochemically, determined by COD_{Cr} , was the highest in column 2 filled with 30 cm thick foam.
3. The changes in hydraulic loading (from 31.8 to 229.2 $\text{mm}\cdot\text{d}^{-1}$) resulted in a variable reduction of ammonium nitrogen. The reduction effectiveness depended on the thickness of the filter layer in columns. A high content of an empty fraction in the columns ensured better transfer of gas, and thus had a positive effect on the oxygen conditions within.
4. Total suspended solids were trapped on the studied beds regardless of the filter bed thickness (differences in mean values in effluents from columns 1 and 2 were statistically insignificant).
5. The results of the Kruskal-Wallis test confirm the significant influence of the filter layer thickness on the dissolved oxygen level and the degree of COD_{Cr} and N-NH_4^+ reduction.
6. Due to the favorable conditions for the filter colonization by biomass, column 2 filled with 30 cm thick foams was the most effective in the removal of BOD_5 , COD_{Cr} , N-NH_4^+ and total suspended solids. For this filter, the average reduction in these pollutants was 94.7%, 93.8%, 84.0% and 68.3%, respectively, under variable hydraulic loading during five months of the experiment.
7. The use of foam trims (with nominal diameters d_{10} and d_{60} of 4 mm and 10 mm, respectively) as biofilter filling supported growth and development of microorganisms to the same high extent as new foam materials installed in plastic sheaths. High porosity of foams (94.6%) ensured a concentration of dissolved oxygen at a level sufficient to remove both organic pollutants and ammonium nitrogen from the sewage.
8. Reuse of PUF waste in the form of trims of upholstery foams as elements of a system consisting of a septic tank/storage tank and a vertical flow biofilter, proved to be an efficient way for direct treatment of sewage. This technology, being uncomplicated, easy to control, and cost effective, as opposed to most of the existing aerobic systems which require additional aeration, offers a highly promising solution for the biological treatment of household sewage.

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